

An educational tool for design and detailing of reinforced concrete columns

Uma ferramenta didática para o dimensionamento e detalhamento de pilares de concreto armado



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Abstract

This text presents a teaching tool developed for the design of reinforced concrete column. The basic functionality and validation results of the application are shown here. A complete example using the expressions of the Brazilian code for the design of reinforced concrete structures, NBR6118:2003, is also presented.

Keywords: applets, column design, reinforced concrete, teaching tools.

Resumo

Este texto apresenta uma ferramenta didática desenvolvida para o dimensionamento e detalhamento de pilares de concreto armado. São apresentadas as funcionalidades básicas do aplicativo e o processo de validação dos resultados obtidos. Um exemplo de dimensionamento de um pilar utilizando as expressões da norma brasileira de projeto de estruturas de concreto, NBR6118:2003, também é apresentado.

Palavras-chave: applets, dimensionamento de pilares, concreto armado, ferramentas de ensino.

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1. Introduction

Multimedia elements as support material in engineering education have been the focus of researchers in Brazil and in other countries as well (Assis & Bittencourt [1]). Java™ *applets*, which are small applications written with the Java™ language that can be executed within a web browser, have been shown to be very useful multimedia elements. This type resource enables the student to have access to a tool capable of making complex calculations through a very simple graphical user interface.

Having access to this type of resource in the classroom is especially useful when studying topics that can only be solved with the help of a computer.

2. Application description

The developed software has the objective of designing the reinforcement area for a simply supported reinforced rectangular concrete column subjected to skew bending. The first step in the program consists of the user inputting the basic loading and geometric data for the problem. The program then does a second order analysis and determines the minimum and final forces for the reinforcement area design. In the next step the reinforcement area is determined for a reinforcement arrangement chosen by the user. The last step consists on the detailing of the reinforcement

section with the reinforcement area that was determined in the last step. As this program is a teaching tool some of these steps require the intervention of the user. In the items below each of these steps will be further described giving a better notion about the applet itself. The reader may access the applet on the Internet in the following address <http://www.lmc.ep.usp.br/pesquisas/tecedu>.

2.1 Data input

The geometry and loading data for the column is inputted into the screen shown in Figure 1. This data includes the dimensions for the rectangular section, the effective length for the column in the x and y directions, the normal force acting on the column, the first order bending moments acting on the ends of the column, the concrete strength, the reinforcement class and the gross cover for the reinforcement.

2.2 Determining second order effects

The routine adopted in this part of the applet uses the formulations presented by the Brazilian Reinforced Concrete Code NBR6118:2003 [2]. This routine checks if second order effects are negligible, if they are not, it calculates the second order effect through a moment magnification equation. Figure 2 shows a screenshot of this calculation process, this way the student may easily check his work if necessary.

Figure 1 - Data input screenshot

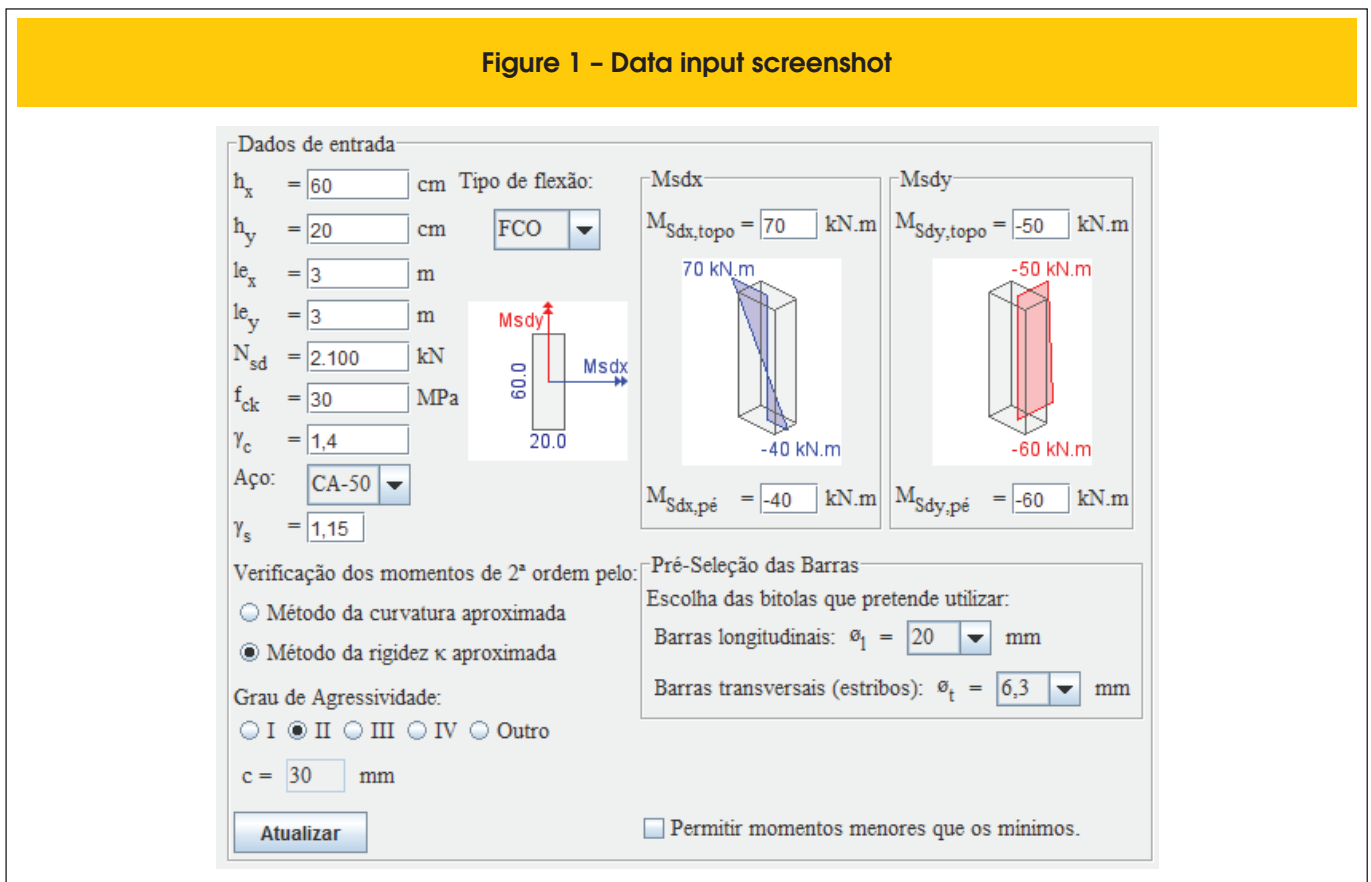
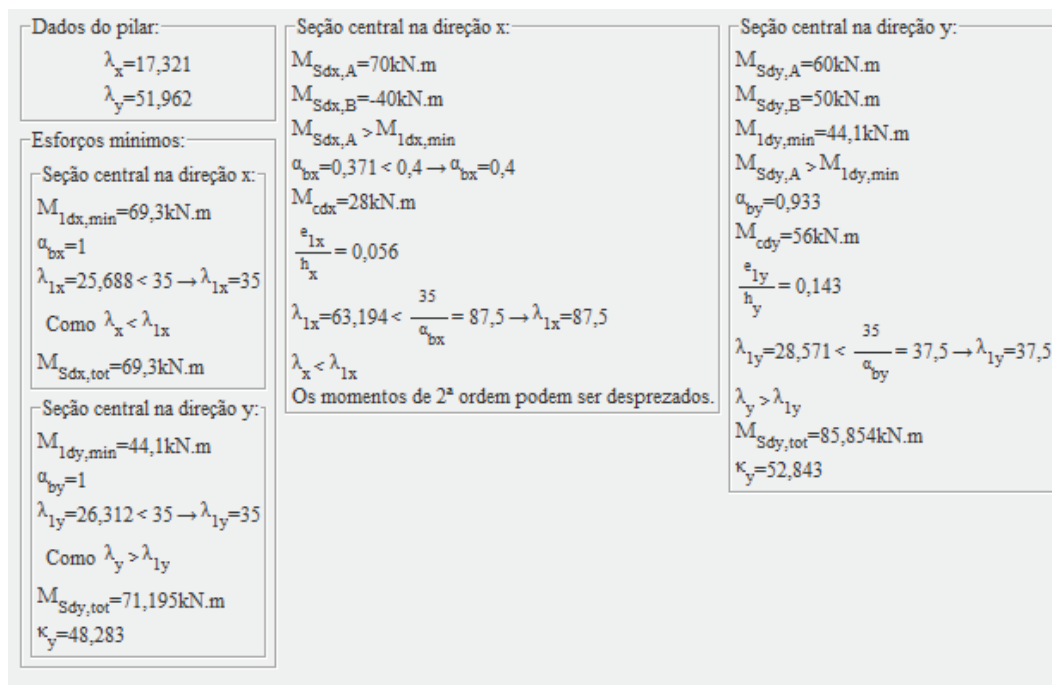


Figure 2 – Design force calculation summary



2.3 Reinforcement area design

In the next stage the user must choose the bending moment pair acting on the column section and the reinforcement distribution on that section. The applet then finds the reinforcement ratio for that section that has a failure surface that passes through the selected acting forces. The algorithms used to find the failure surface can be found in Santos [3]. Some changes on these algorithms had to be made in order to integrate them into an object oriented programming structure and also some changes were made to improve performance, these changes include: use of a *Newton-Raphson* procedure to find the neutral axis depth and the an adaptive *Simpson* method for the numerical integrations necessary throughout the process.

As was mentioned in the last paragraph the user must choose the reinforcement distribution in the cross section of the column, Figure 3 shows some of the distributions available to the user. The gross cover used to position the reinforcement layers shown is determined from the data the user inputted in the first tab.

After a reinforcement ratio value is found the user may cycle through the acting forces that the column must resist and see if they are inside of the failure curve, Figure 4 shows the failure curve for a given ratio and the possible acting forces.

2.4 Reinforcement detailing

Given the reinforcement ratio calculated in the previous step, the longitudinal rebar diameter and the reinforcement distribution, the program automatically distributes de rebars in the section (Figure

5). The applet checks if the spacing satisfies the specified rules given by the NBR6118:2003 code, giving a warning to the user in case the spacing is inadequate. Having defined the longitudinal reinforcement, the user must then manually place the necessary stirrups. The applet automatically checks if any rebars are not properly tied painting the rebar in a red color (Figure 6(a)). A cross section properly tied by stirrups is shown in Figure 6(b).

3. Results

3.1 Validation of the failure surface

The validation of the failure surfaces generated by the applet was done through comparisons with results presented by Smaniotto [4] and Montoya et. al. [5]. Smaniotto [4] presented a tool for automatic design of column cross-sections in biaxial bending. For validation of his work he demonstrated a comparison with the results, for the cross-section shown in Figure 7, from other programs. Figure 8 shows the moment diagrams for a series of normal force values, here PDOP represents the results for the program by Smaniotto [4], Eberick is a commercial program for building design, nFOCCA is a program created by Santos [3] and *applet* represents the results obtained with the tool here presented.

Montoya et. al. [5] presented several interaction diagrams for an array of cross-section types, reinforcement distributions and ratios. These diagrams are commonly used for the manual design of columns in Brazil. For data validation purposes the results from the *applet* were plotted over some of these diagrams Figures 9 and 10 show these results.

3.2 Design example

In order to give a better understanding of the applet itself a problem from the Technical Commentaries and Examples from the Brazilian Code (IBRACON [6]) will be solved. The problem consists on finding the reinforcement for the column presented in Figure 11.

For comparison purposes the design forces for the problem in IBRACON [6] are presented in Table 1. For these design forces it was proposed a cross section with stirrups of 6,3 mm and concrete cover of 30 mm and longitudinal reinforcement consisting of fourteen 20 mm rebars, as shown in Figure 12.

Inputting the data from Figure 11 into the *applet*, gives the results shown in Figure 2, the design forces from this screen are summarized in Table 2. Comparing the results from the two tables it shows a difference in the values for the moment in the middle of the column. This difference is due the calculation of the 2nd order effects in the x direction. In the example the 2nd order effects is always calculated in both directions if these effects are significant in even only one direction. The Brazilian Code does not mention this practice in its text and therefore the applet only finds the 2nd order moment in the directions that have significant 2nd order effects. As can be seen this practice does not change the final results because the additional 2nd order moment in the x direction is small.

The reinforcement ratio necessary for these acting forces is presented in Figure 4, with the chosen distribution and this reinforcement ratio the program arrives in a cross-section consisting of twelve 20 mm rebars, the final cross-section is shown in Figure 5. Failure curves were generated for both proposed cross-sections and the design forces were plotted with these curves in Figure 13. This figure shows that both solutions are compatible and that the difference in design force evaluation did not affect the resulting cross-section.

4. Conclusions

The presented tool is a practical solution for designing reinforced concrete columns. If used as an educational tool, it gives the student access to a very powerful computing tool replacing the use of interaction diagrams that end up being hardly used in the day to day in engineering design.

The validation presented shows that the *applet* provides consistent results when compared to other programs and to interaction diagrams from the literature. Some divergence in the results appear in Figure 8 for normal forces of 3600 kN, these were noted also by Smaniotto[4], and the greater resisting moments shown by the Eberick program are explained by the author as differences in the method of calculation of the resultant of the concrete compression field. Santos [3] states that numerical precision is guaranteed when using numerical integration for normal load values less than 99% of the total capacity of the cross-section.

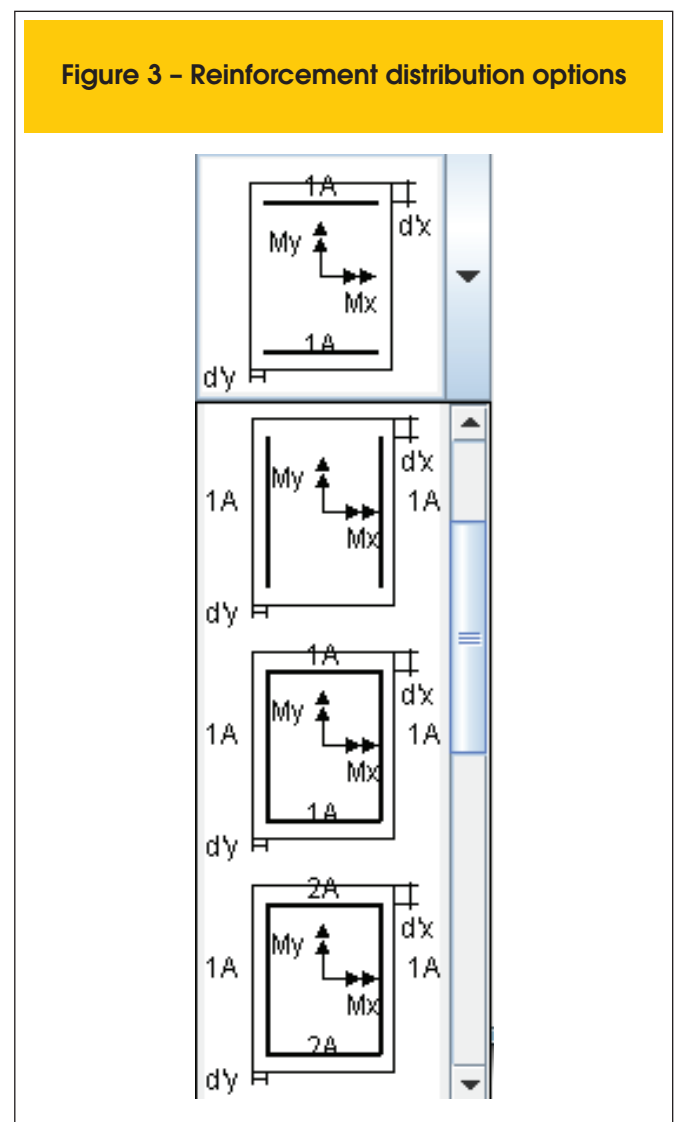
The use of interaction diagrams for reinforcement design in columns is less exact than using the *applet*. This is because these diagrams are generated for gross cover values that are not the real gross cover and therefore corrections to the reinforcement values have to be made. Another fact that create even more inaccuracies is the fact that, in most cases, it is necessary to interpolate in between diagrams with different normal values and reinforcement ratio. In the presented design example, it is not clear what technique was used for determining the reinforcement in the cross-

section but the result from the *applet* used less reinforcement and attended the acting forces.

5. References

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Figure 3 – Reinforcement distribution options



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Figure 4 – Reinforcement design screenshot

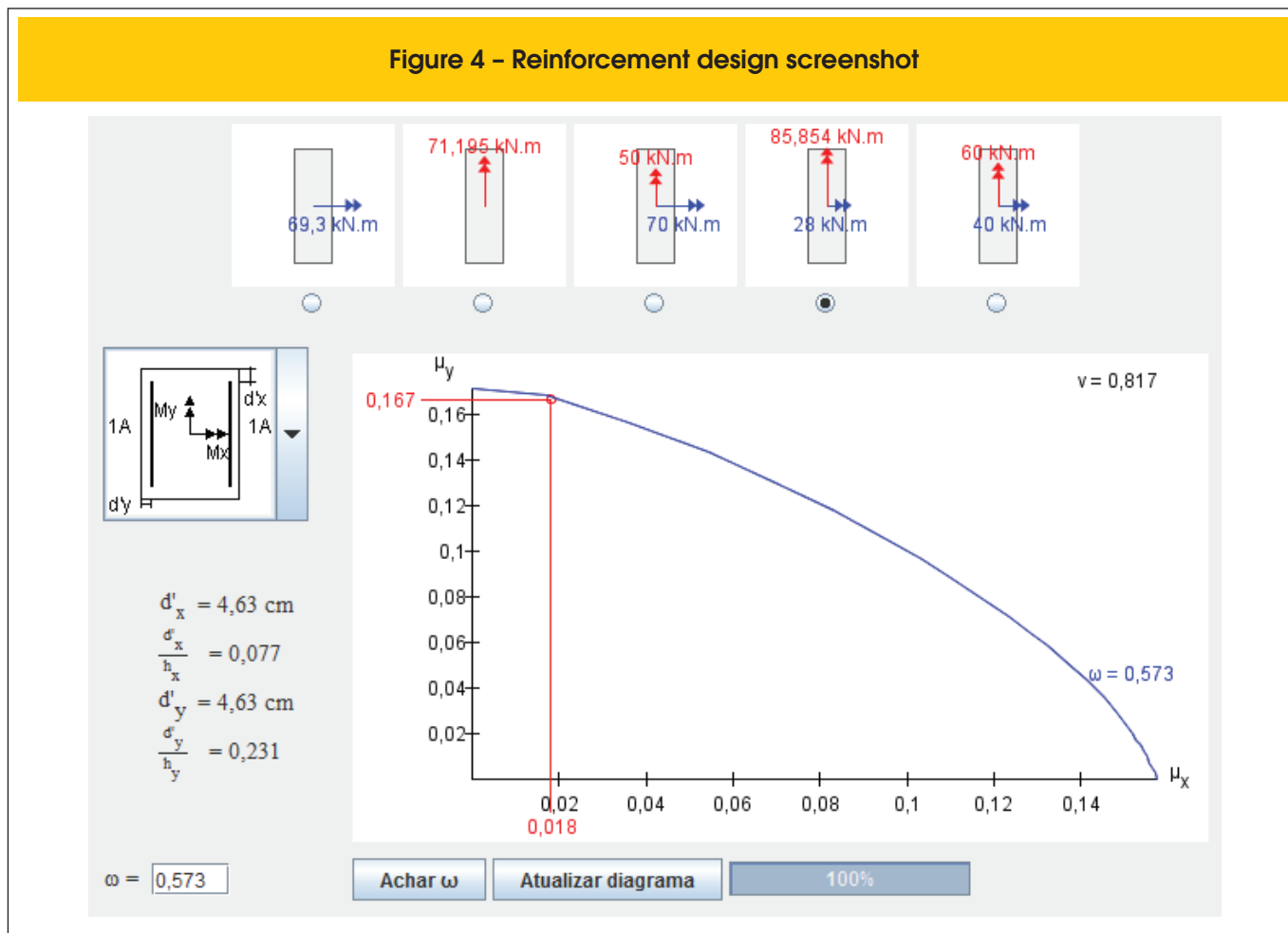


Figure 5 - Cross-section detailing screen

$\omega = 0,5735 \rightarrow A_s = 33,9176 \text{ cm}^2$
 $d'_x = 4,63 \text{ cm}$ $d'_{x,ef} = 4,63 \text{ cm}$
 $d'_y = 4,63 \text{ cm}$ $d'_{y,ef} = 4,63 \text{ cm}$
 $A_{s,corrigido} = 33,9176 \text{ cm}^2$
 $10 \text{ mm} \leq \phi_1 \leq 25 \text{ mm}$
 $\phi_1 = 20 \text{ mm}$
 $n_{total} = 12$
 $\phi_t \geq 5 \text{ mm}$
 $\phi_t = 6.3 \text{ mm}$

$n_x = 0$ $S_{lx} = 10,74 \text{ cm}$

$8 \text{ cm} \leq S_1 \leq 40 \text{ cm}$
 $S_t \leq 20 \text{ cm}$
 $7,245 \text{ cm}^2 \leq A_s \leq 96 \text{ cm}^2$
 $A_{s(real)} = 37,8 \text{ cm}^2$
 $A_{s(emendas)} = 75,6 \text{ cm}^2$

$n_y = 4$
 $S_{ly} = 10,148 \text{ cm}$

Disposição automática

Travamentos

Barra com 2 ganchos

Envolve Estribo

Figure 6 - Rebar with inadequate ties (a) e tied rebars (b)

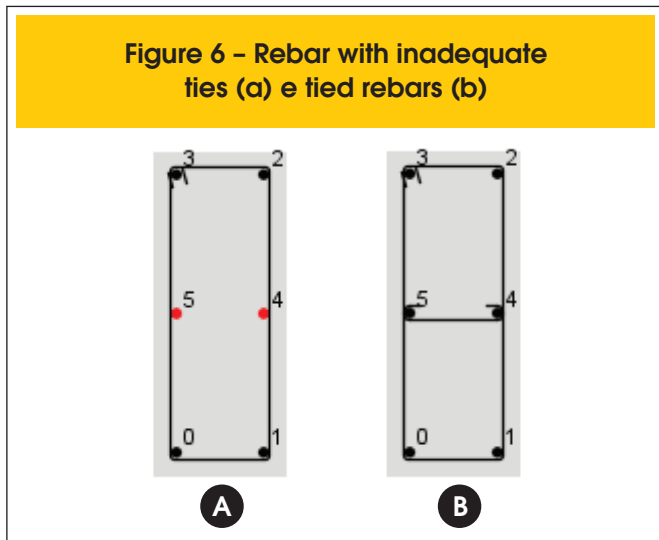


Figure 7 - Cross-section analysed by Smaniotto (4)

$Aço \text{ CA-50}$
 $f_{ck} = 30 \text{ MPa}$
 $\Phi = 10,0 \text{ mm} (0,7854 \text{ cm}^2)$
 $\Phi_t = 5,0 \text{ mm}$
 $34 \Phi 10\text{mm} (26,7 \text{ cm}^2)$
 $\text{Cobrimento} = 2,5 \text{ cm}$

$N1 = 34 \Phi 10,0\text{mm}$

Figure 8 - Comparison of results from applet with other programs (Smaniotto (4))

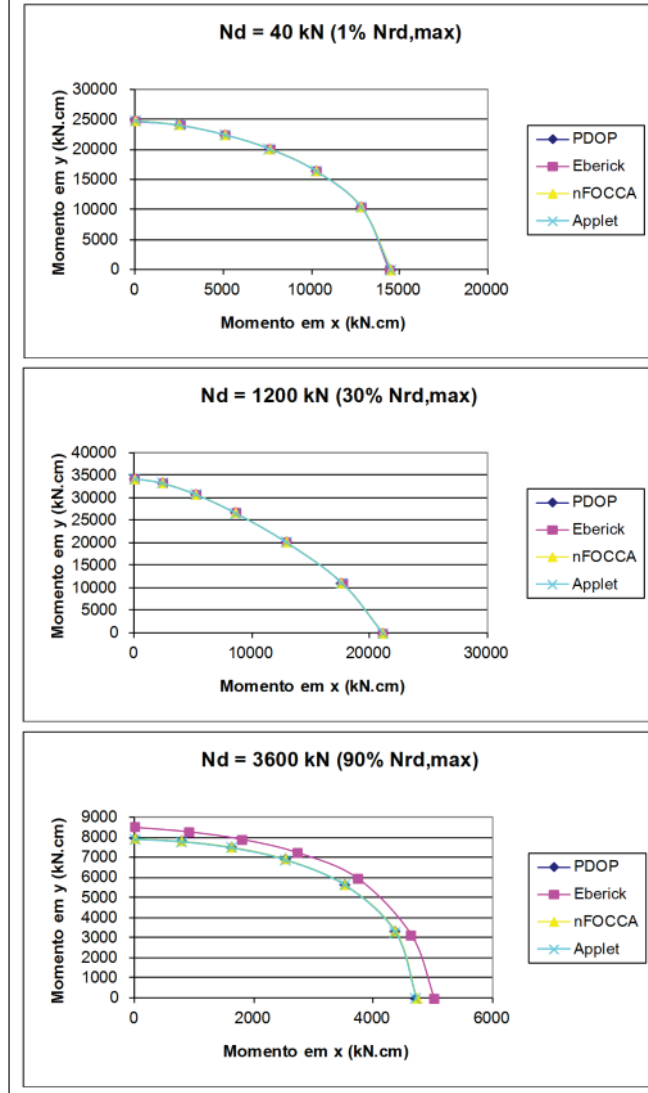


Figure 9 - Comparison of results with interaction diagrams for uniaxial bending (Montoya et al. (5))

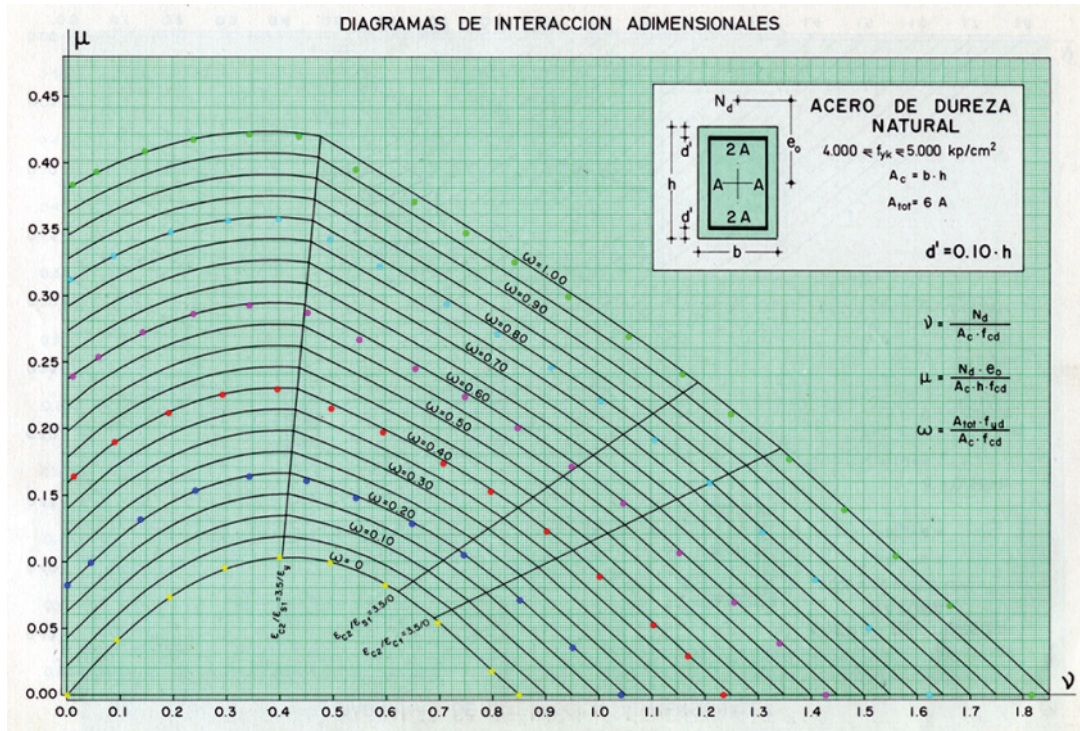


Figure 10 - Comparison of results with interaction diagrams for biaxial bending (Montoya et al. (5))

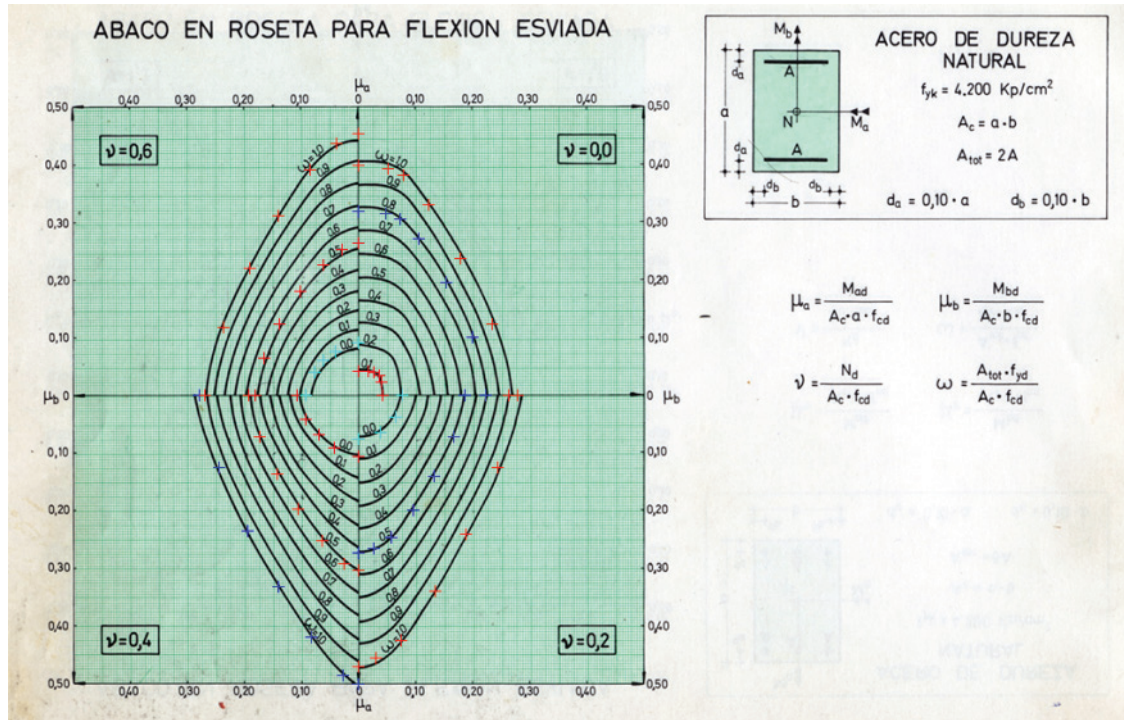


Figure 11 - Data for the example column (IBRACON (6))

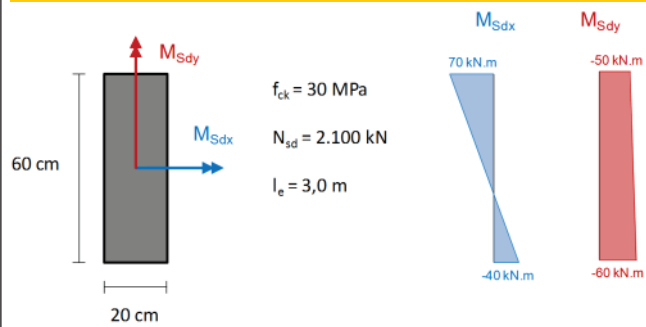


Figure 12 - Designed cross-section from the IBRACON (6) example

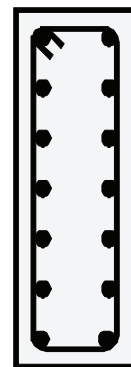


Figure 13 - Failure curves for the proposed cross-sections

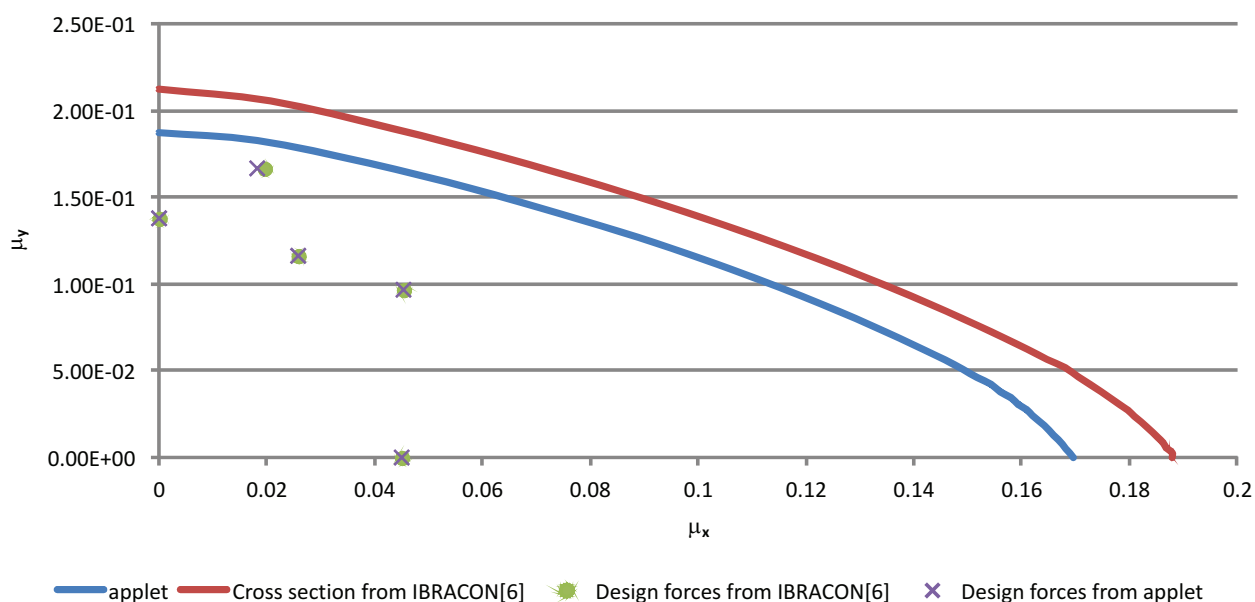


Table 1 - Design forces for the example (IBRACON (6))

	Minimum x-dir	Minimum y-dir	Top moment	Middle moment	Base moment
Mx	69,3 kN.m	-	70 kN.m	30,1 kN.m	-40 kN.m
My	-	71,2 kN.m	-50 kN.m	-85,9 kN.m	-60 kN.m

Table 2 - Design forces for the applet

	Minimum x-dir	Minimum y-dir	Top moment	Middle moment	Base moment
Mx	69,3 kN.m	-	70 kN.m	28 kN.m	-40 kN.m
My	-	71,2 kN.m	-50 kN.m	-85,9 kN.m	-60 kN.m